



Politecnico di Milano  
Department of Structural Engineering

# ***SPECTRAL ELEMENTS FOR THE ASSESSMENT OF THE SEISMIC VULNERABILITY OF STRUCTURES***

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Aussois

# ***GEO-ELSE***

## ***(GEO-ELasticity by Spectral Elements)***

- GEO-ELSE is a Spectral Elements code for the study of wave propagation phenomena in 2D or 3D complex domain
- Developers:
  - CRS4 (Center for Advanced, Research and Studies in Sardinia)
  - Politecnico di Milano, DIS (Department of Structural Engineering)
- Native parallel implementation
- Naturally oriented to large scale applications ( > at least  $10^6$  grid points)

# *Why using spectral elements ?*

- ◆ Suitable for modelling a variety of physical problems (acoustic and elastic wave propagation, thermo elasticity, fluid dynamics)

- ◆ Accuracy of high-order methods

$$\left\| \bar{u} - u_{h,N} \right\| \leq C h^N e^{-N}$$

- ◆ Suitable for implementation in parallel architectures
- ◆ Great advantages from last generation of hexahedral mesh creation program (e.g.: CUBIT, Sandia Lab.)

# *Formulation of the elastodynamic problem*

Dynamic equilibrium in the weak form:

$$\frac{\partial^2}{\partial t^2} \int_{\Omega} \rho u_i v_i d\Omega + \int_{\Omega} \sigma_{ij} \varepsilon_{ij} d\Omega = \int_{\Gamma} t_i v_i + \int_{\Omega} f_i v_i$$

where  $u_i$  = unknown displacement function

$v_i$  = generic admissible displacement function (test function)

$t_i$  = prescribed tractions at the boundary  $\Gamma$

$f_i$  = prescribed body force distribution in  $\Omega$



## *Time advancing scheme*

Finite difference 2<sup>nd</sup> order (LF2 – LF2)

$$\begin{cases} \frac{\partial^2 u}{\partial t^2} = \frac{u_{n+1} - 2u_n + u_{n-1}}{\Delta t^2} \\ \frac{\partial u}{\partial t} = \frac{u_{n+1} - u_{n-1}}{2\Delta t} \end{cases}$$

Courant-Friedrichs-Levy (CFL) stability condition

$$\Delta t \leq \eta \left( \frac{\Delta x}{c} \right)_{\min}$$

## *Spatial discretization*

Spectral element method SEM (Faccioli et al., 1997)

# ***Bibliography***

Kosloff D, Baysal E.

*Forward modelling by the Fourier method*

**Geophysics 1982; 47: 1402-1412**

Kosloff D, Kessler D, Filho AQ, Tessmer E, Behle A, Strahilevitz R.

*Solutions of the equations of dynamics elasticity by a Chebyshev spectral method*

**Geophysics 1990; 55: 748-754**

Faccioli E, Maggio F, Paolucci R, Quarteroni A.

*2D and 3D elastic wave propagation by a pseudo-spectral domain decomposition method*

**Journal of Seismology 1997; 1 237-251.**

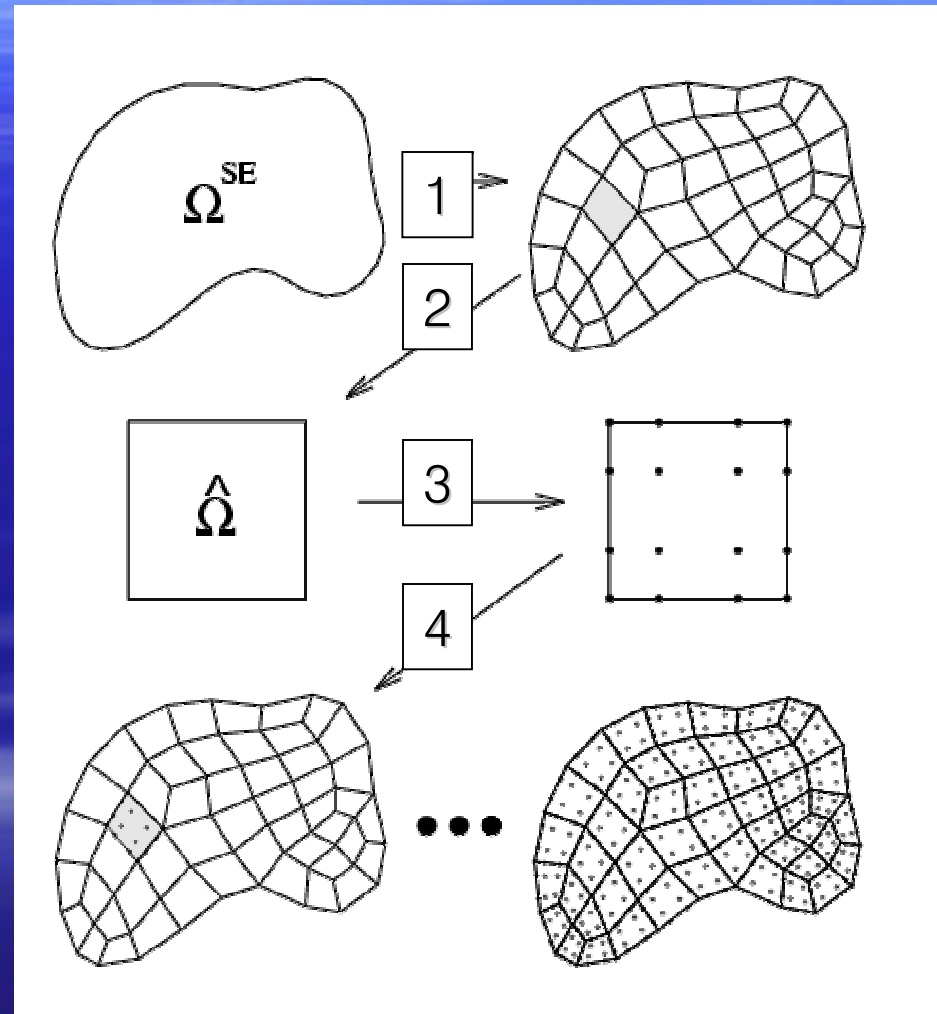
Komatitsch D, Vilotte J-P.

*The spectral element method: an efficient tool to simulate the seismic response of 2D and 3D geological structures.*

**Bull. Seism. Soc. Am. 1998; 88: 368-392.**

# *Spectral discretization of the spatial domain*

- ◆ The domain is split into quadrilaterals (hexahedra)
- ◆ Each subdomain is mapped onto a reference element
- ◆ LGL nodes are introduced
- ◆ Spectral grid-points are mapped back onto the domain



# *The Legendre-Gauss-Lobatto quadrature formula*

$$\int_{-1}^1 f(x) dx \cong \sum_{k=0}^N \alpha_k f(x_k)$$

where

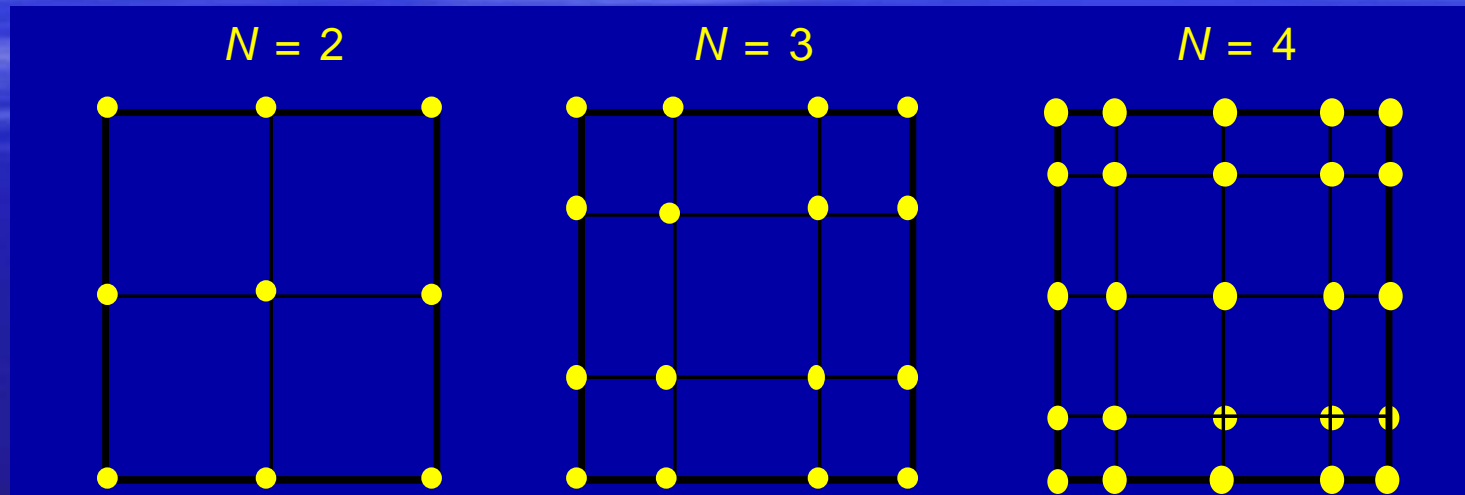
$$\alpha_k = \frac{2}{N(N+1)} \frac{1}{L_N^2(x_k)}$$

$L_N(x_k)$  being the Legendre orthogonal polynomial of degree  $N$ , calculated at the LGL node  $x_k$

LGL nodes:  $L'_N(x_k) = 0$  [ $x_0 = -1, \dots, x_N = 1$ ]



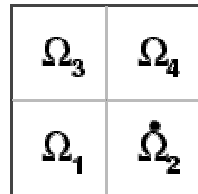
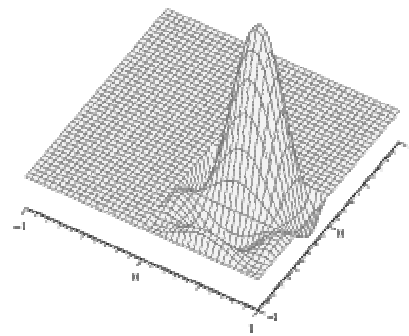
# *2D spectral elements and LGL nodes for different values of the polynomial degree*



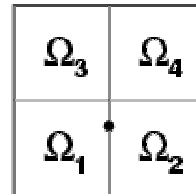
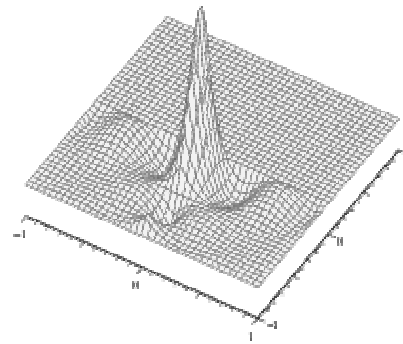
# *Selection of the test functions*

A suitable choice is the Lagrange polynomial of degree  $N$ , which is equal to one at the  $j^{th}$  LGL node and vanishes at all other nodes

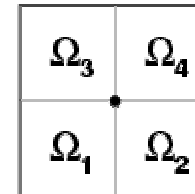
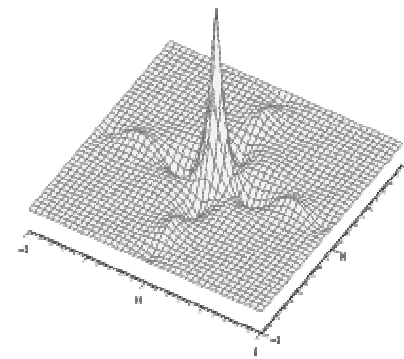
Internal point



Interface point



Cross-point



# *FEM/SEM comparison*

## **Convergence of the method**

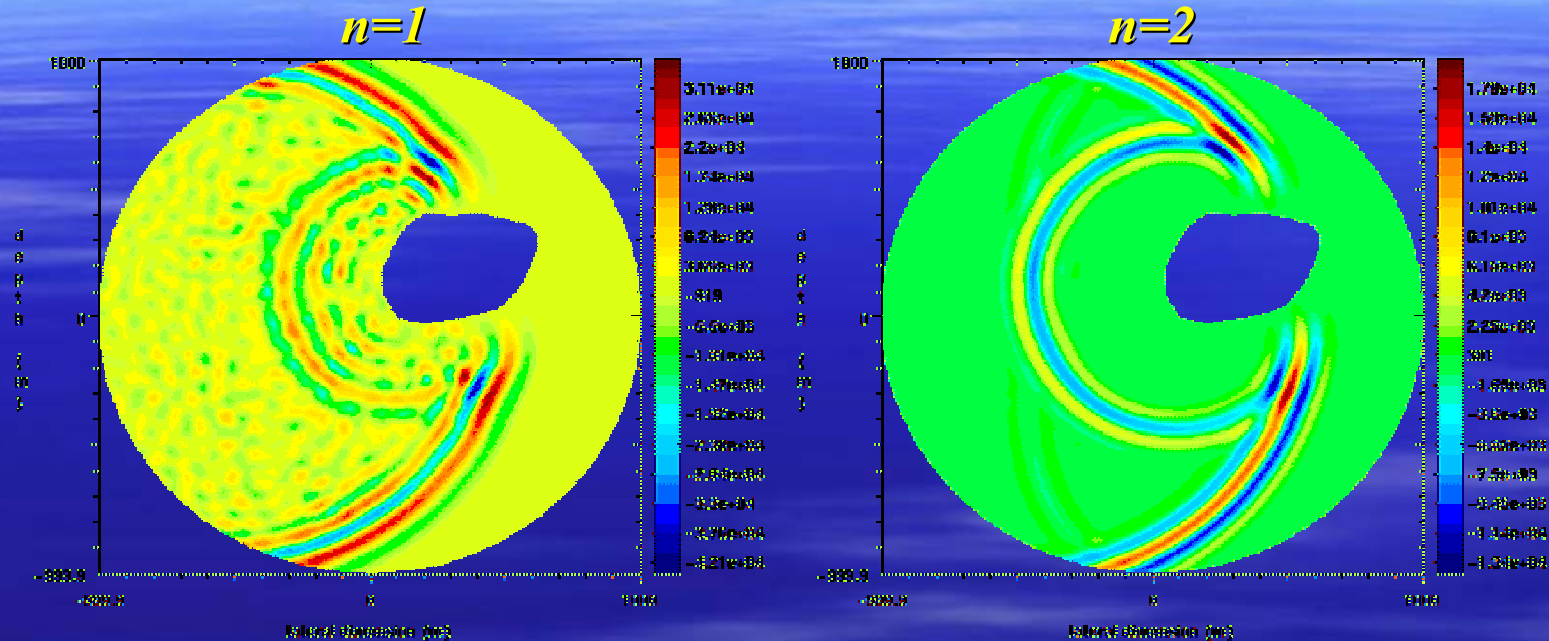
$$\text{FEM} \rightarrow \left\| \bar{u} - u_{h,N} \right\| \leq C h^N$$

$$\text{SEM} \rightarrow \left\| \bar{u} - u_{h,N} \right\| \leq C h^N e^{-N}$$

The SEM converges more rapidly than FEM or, equivalently, it is more accurate for a given number of nodal points (spectral accuracy)

# Why using spectral elements ?

## acoustic problem #1



Acoustic wave propagation through an irregular domain. Simulation with spectral degree 1 (*left*) exhibits numerical dispersion due to poor accuracy.

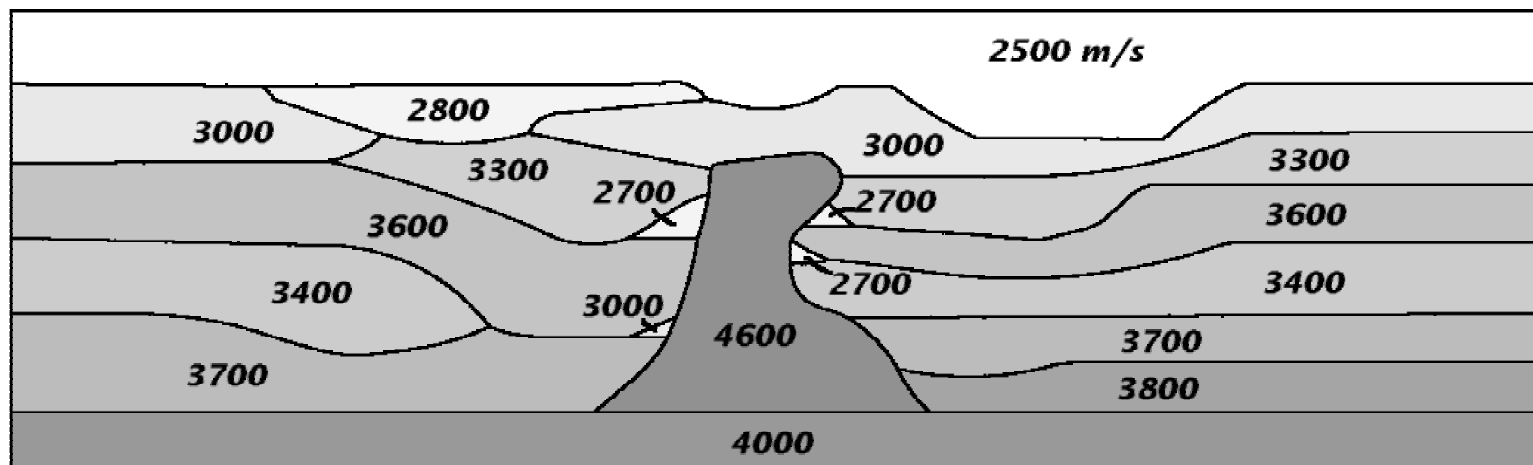
Simulation with spectral degree 2 (*right*) provides better results. Change of spectral degree is done at **run time**.



# *Why using spectral elements ?*

## acoustic problem #2

### Domain geometry

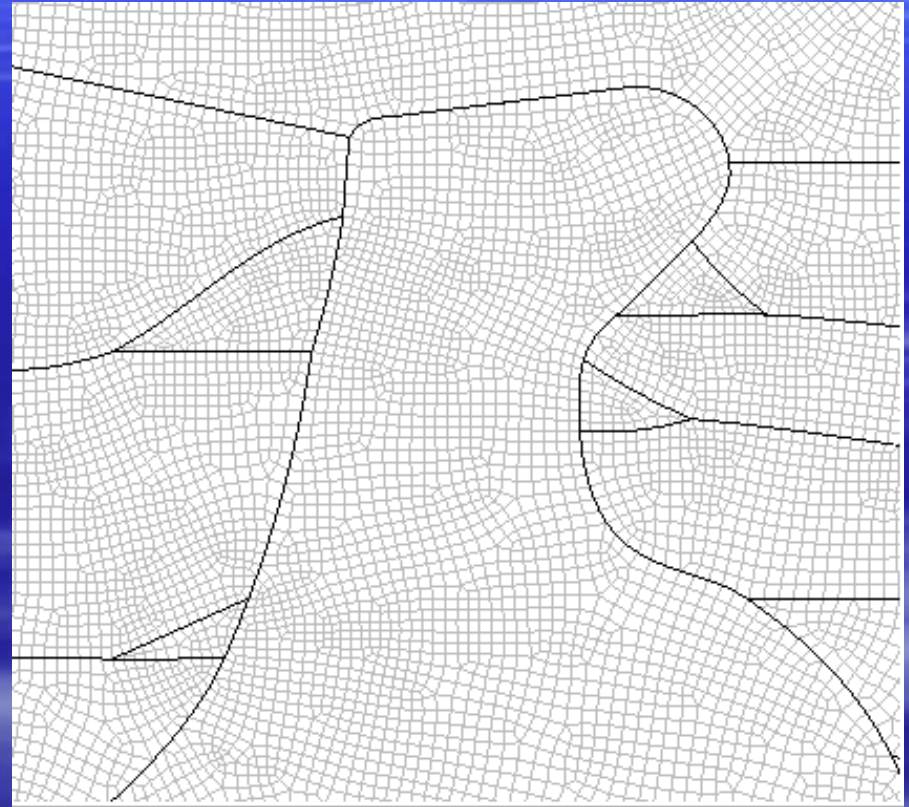


*Courtesy of Dr. F. Maggio*

# *Why using spectral elements ?*

## acoustic problem #2

- Fully unstructured mesh (~ 39000 quads)
- Spectral degree  $n=4$
- ~ 618000 spectral grid-points

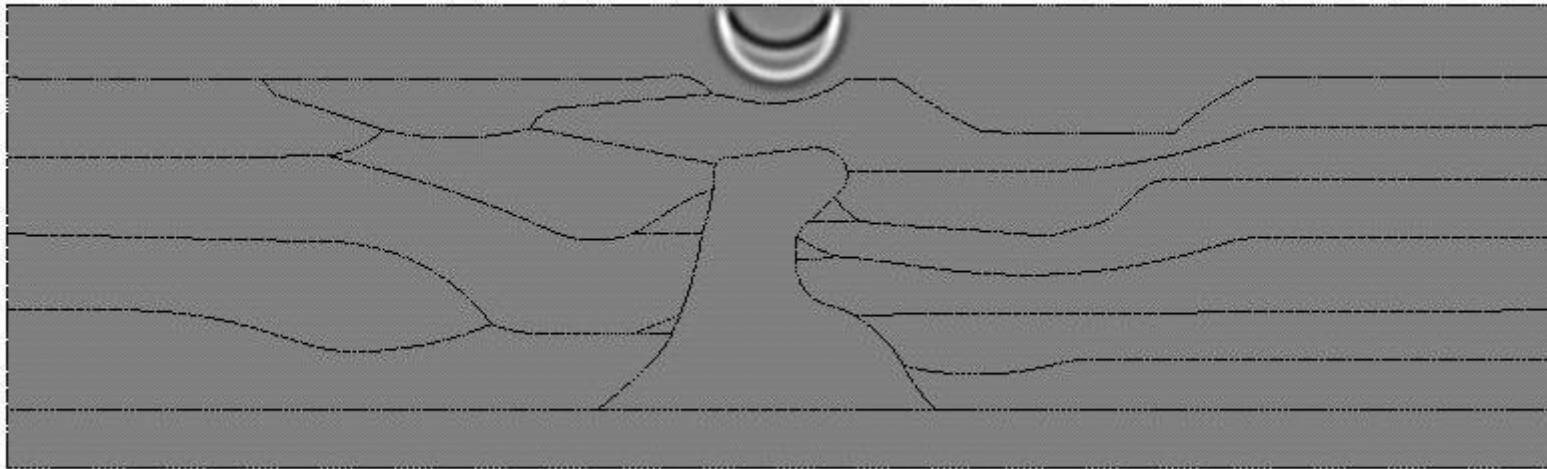


*Courtesy of Dr. F. Maggio*

## *Why using spectral elements ?*

### acoustic problem #2

- Snapshots are taken every 0.25 s, up to 2.00 s
- Pressure values are normalized to  $[-1,+1]$  (from black to white).



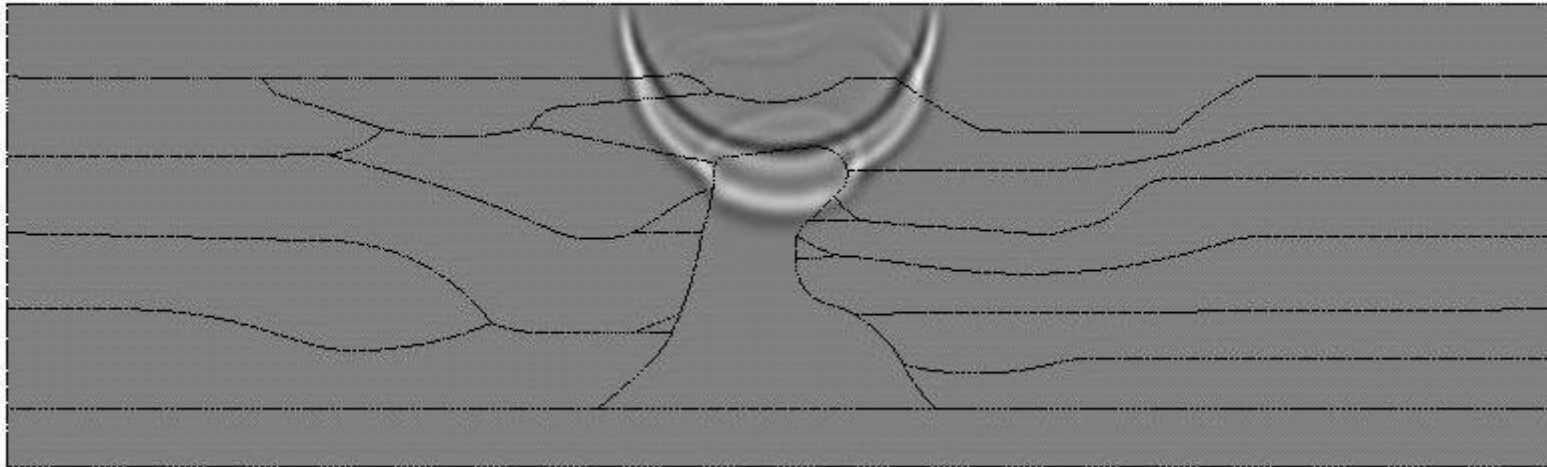
*Courtesy of Dr. F. Maggio*



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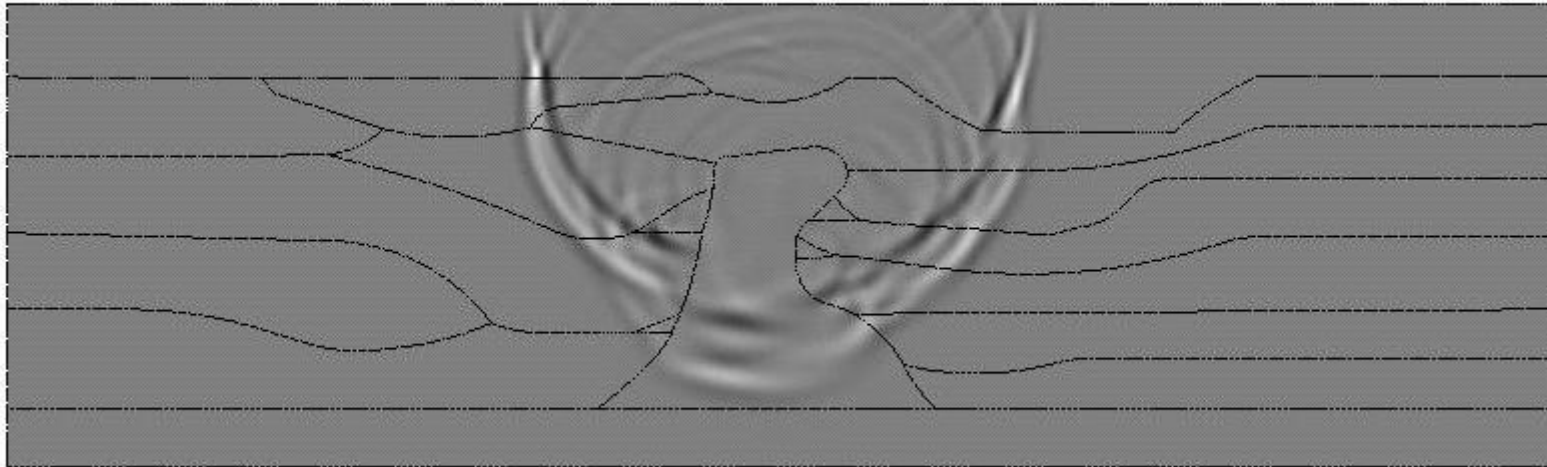
*Courtesy of Dr. F. Maggio*



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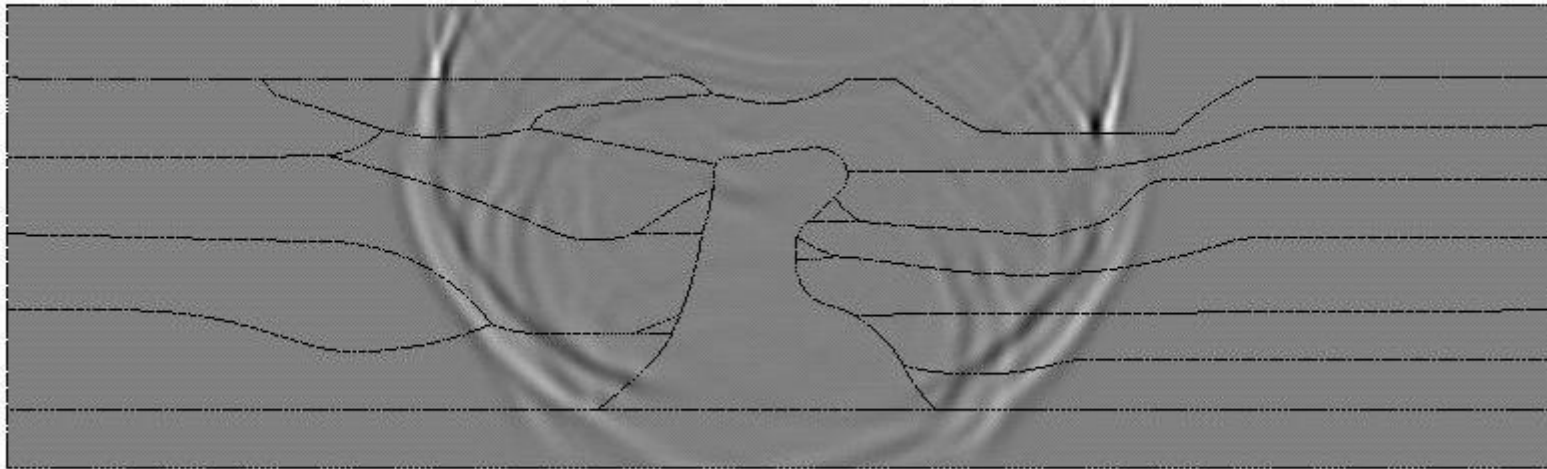


*Courtesy of Dr. F. Maggio*

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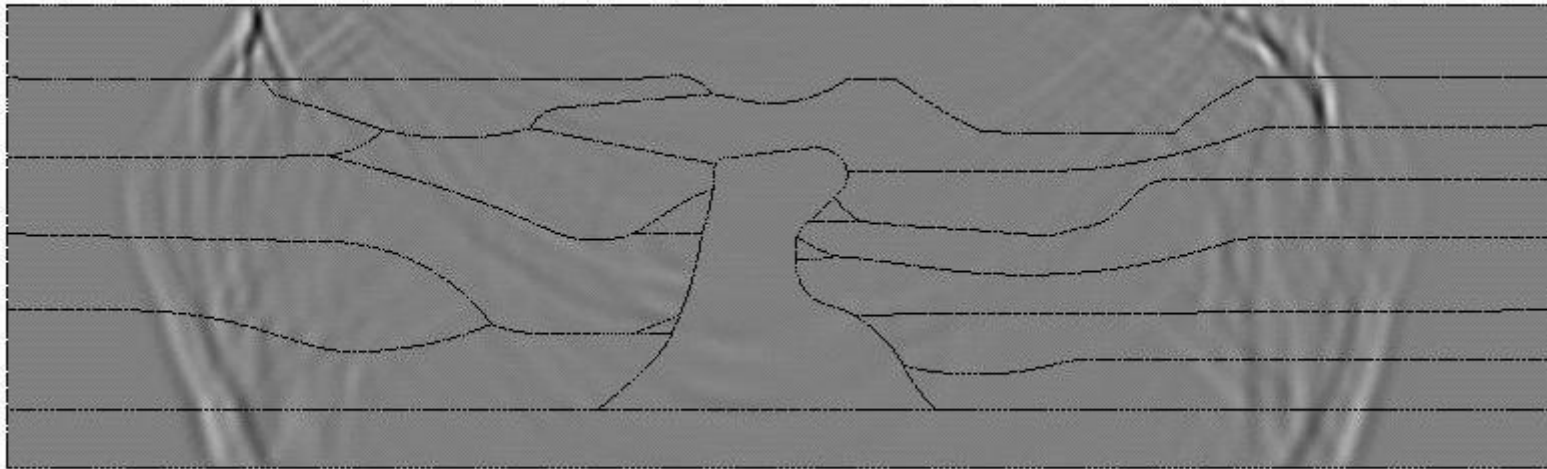


*Courtesy of Dr. F. Maggio*

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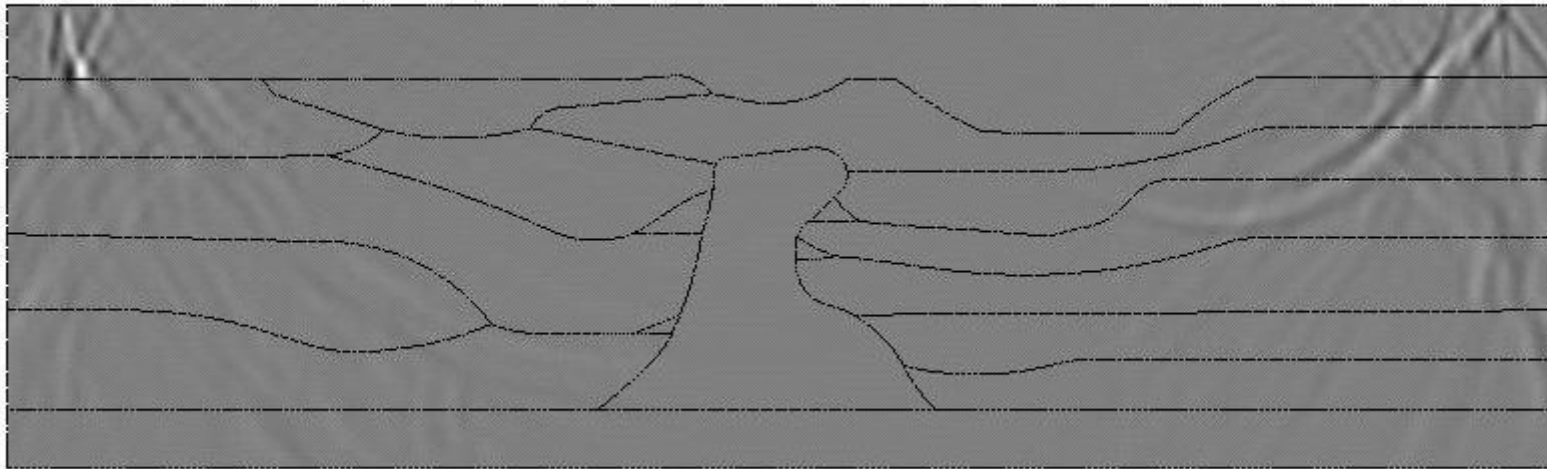
*Courtesy of Dr. F. Maggio*



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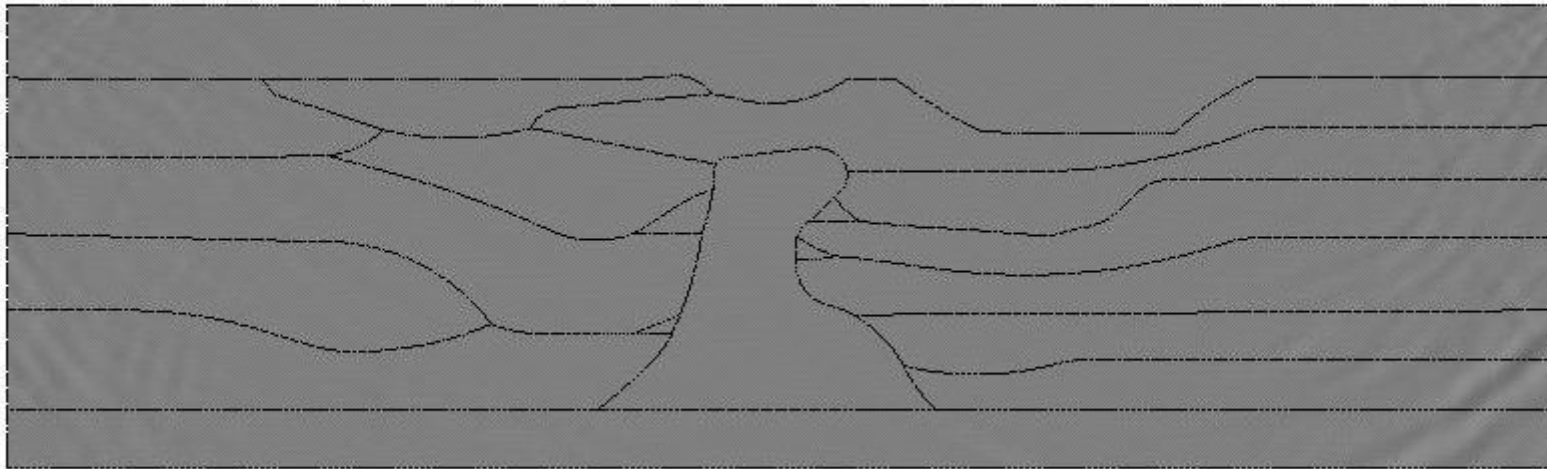
*Courtesy of Dr. F. Maggio*



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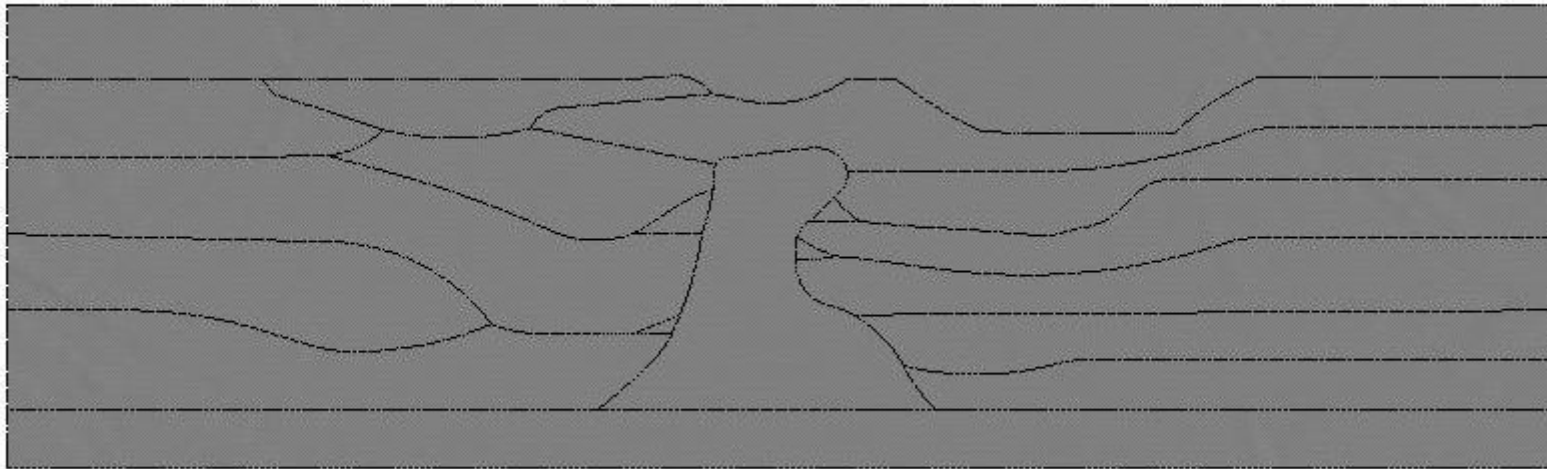


*Courtesy of Dr. F. Maggio*

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*Courtesy of Dr. F. Maggio*

# *FEM/SEM comparison*

## Mass matrix

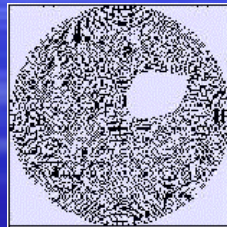
FEM → Non diagonal and symmetric

SEM → Diagonal

SEM coupled with an explicit time approximation scheme is very effective in term of CPU time saving



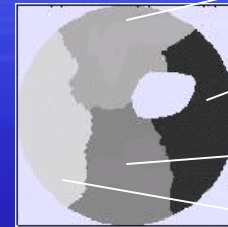
# *Parallel Strategy adopted in GEO-ELSE*



Computational  
domain partitioning  
(METIS, University of  
Minnesota, USA)



Algebraic system  
solution  
(AZTEC, Sandia  
National  
Laboratories, USA)



CPU 1

CPU 2

CPU 3

CPU 4

$A^{(1)}U=b$

$A^{(2)}U=b$

$A^{(3)}U=b$

$A^{(4)}U=b$

$$\text{Parallel Efficiency} = T_{\text{seq}} / (N_{\text{CPU}} \cdot T_{\text{par}}) \approx 90\%$$

*Courtesy of Dr. F. Maggio*

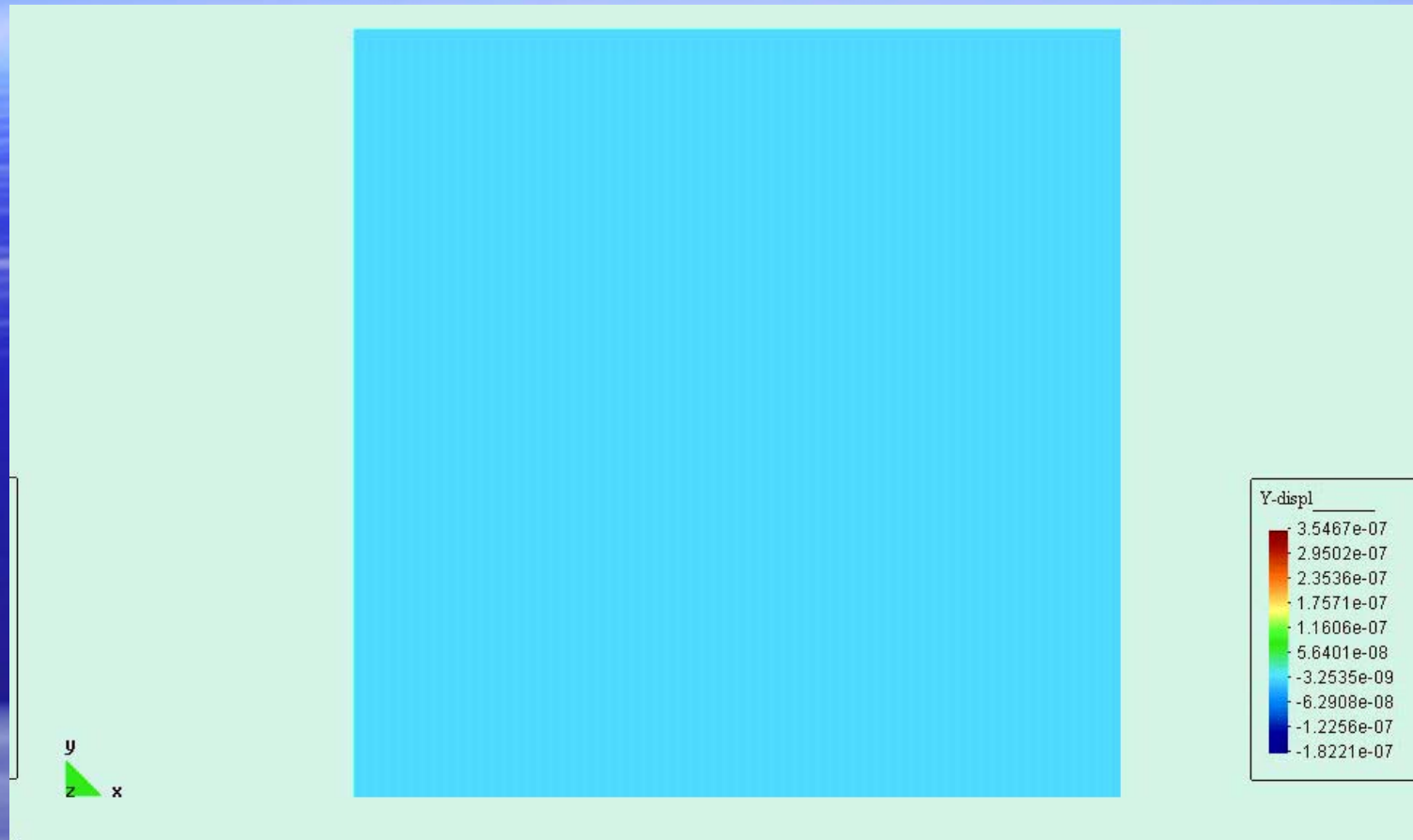


# *Absorbing boundaries*

Stacey 1<sup>st</sup> order (1988, “improved” paraxial conditions)



# Viscoelastic damping



Modified wave equation

$$\frac{\partial^2 u}{\partial t^2} \Rightarrow \frac{\partial^2 u}{\partial t^2} + 2\gamma \frac{\partial u}{\partial t} + \gamma^2 u$$

Q factor, function of the frequency

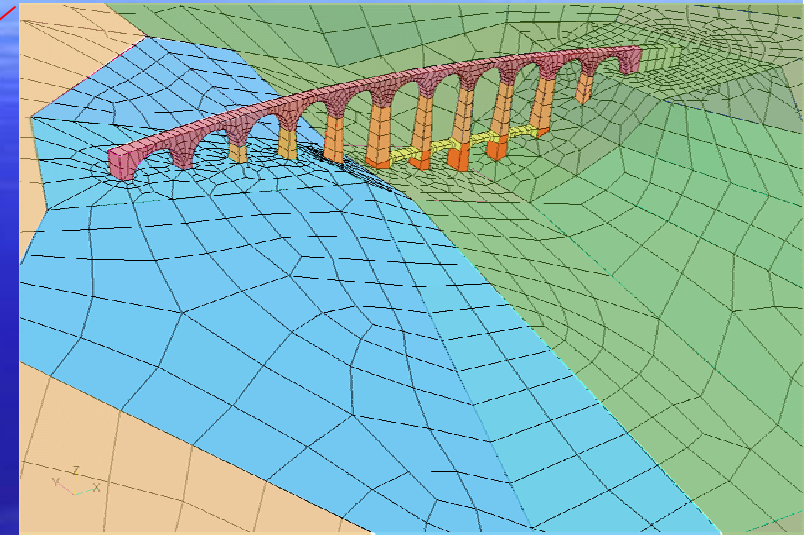
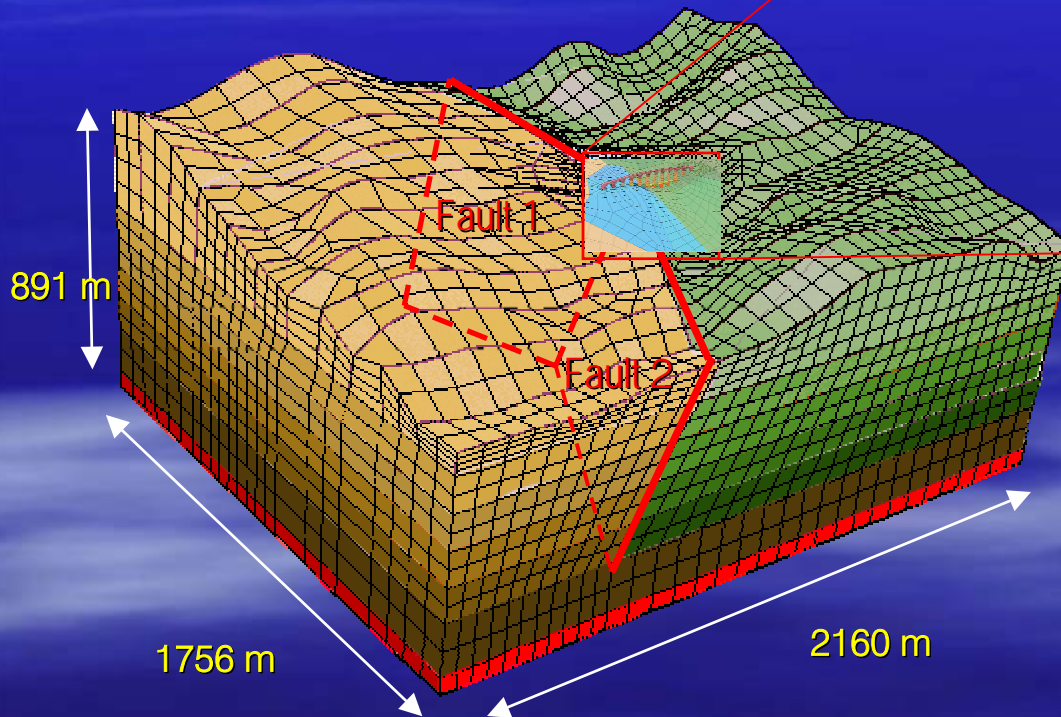
$$Q(f) = \frac{\pi}{\gamma} f$$

# *3D Soil-Structure - Acquasanta viaduct*



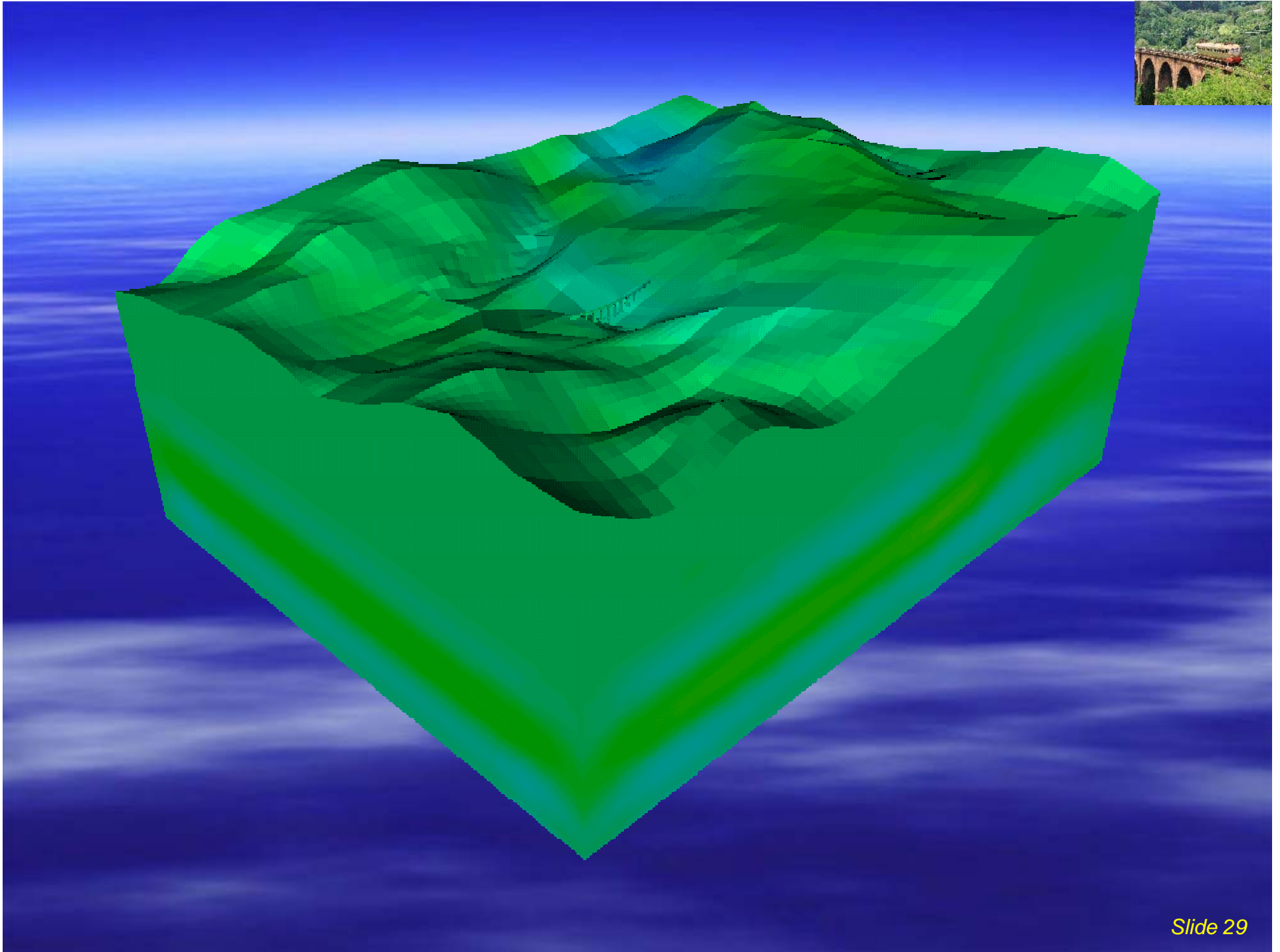
# 3D Soil-Structure - Acquasanta viaduct

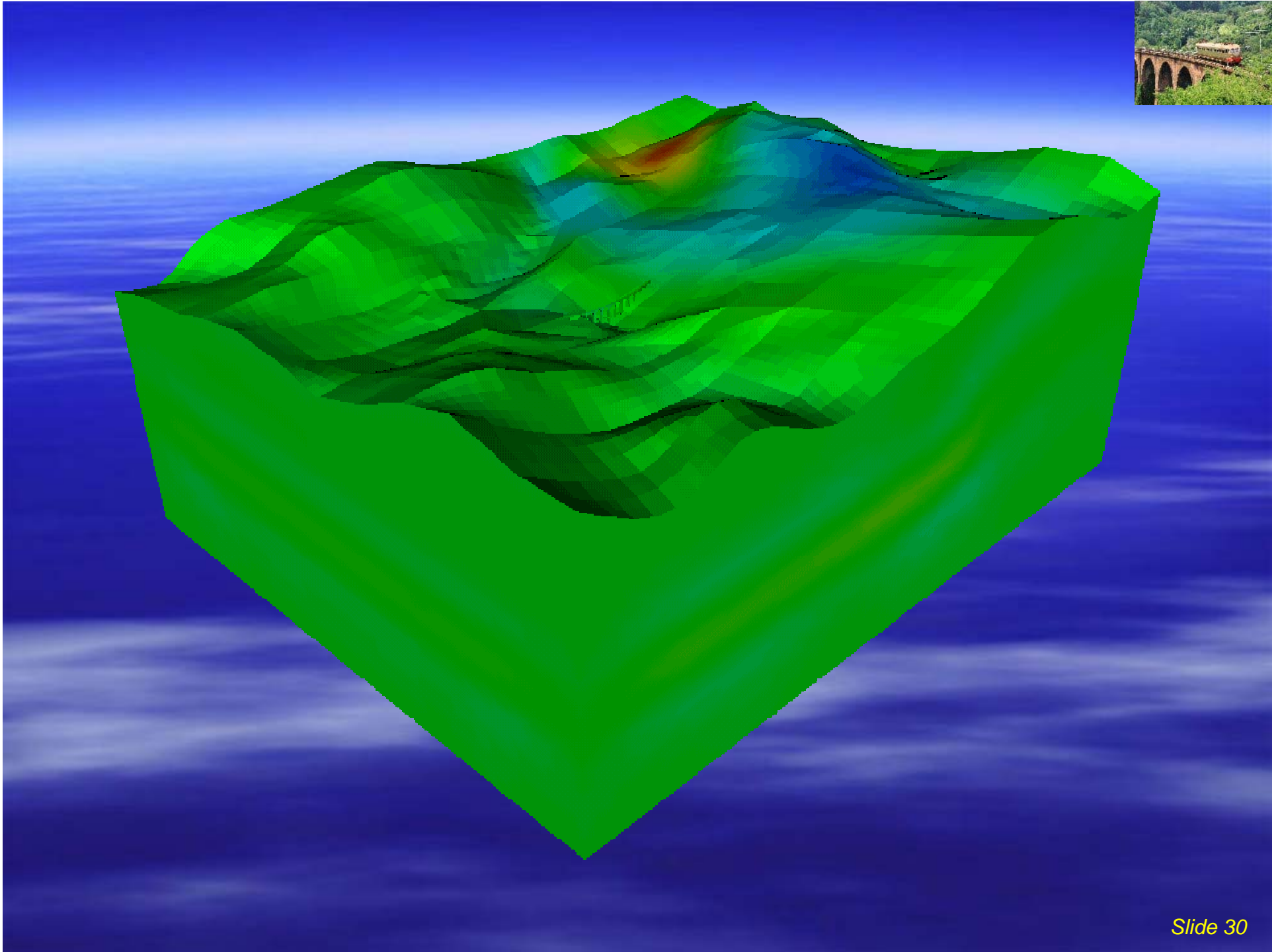
Complex 3D layered structures  
with two main faults

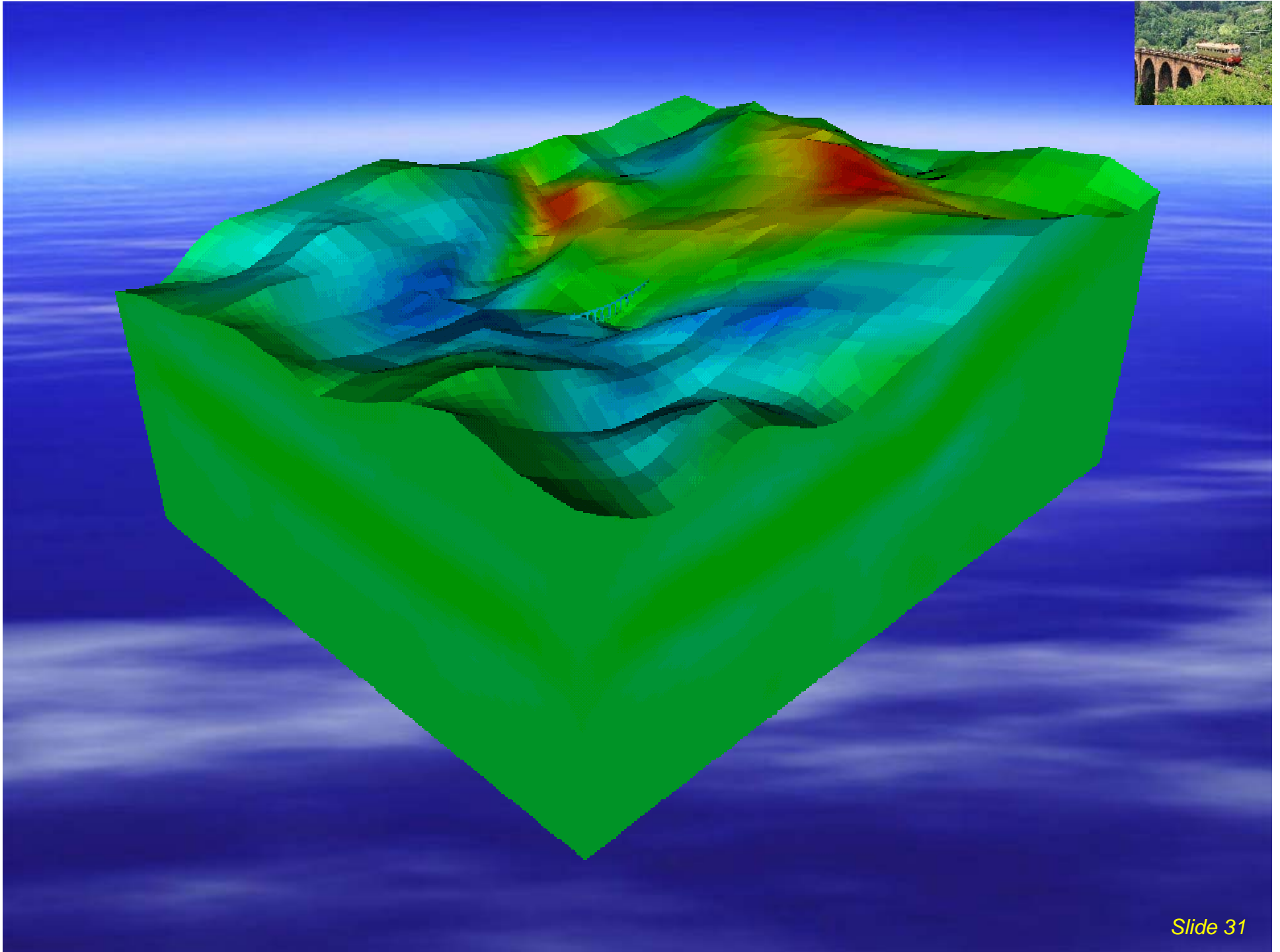


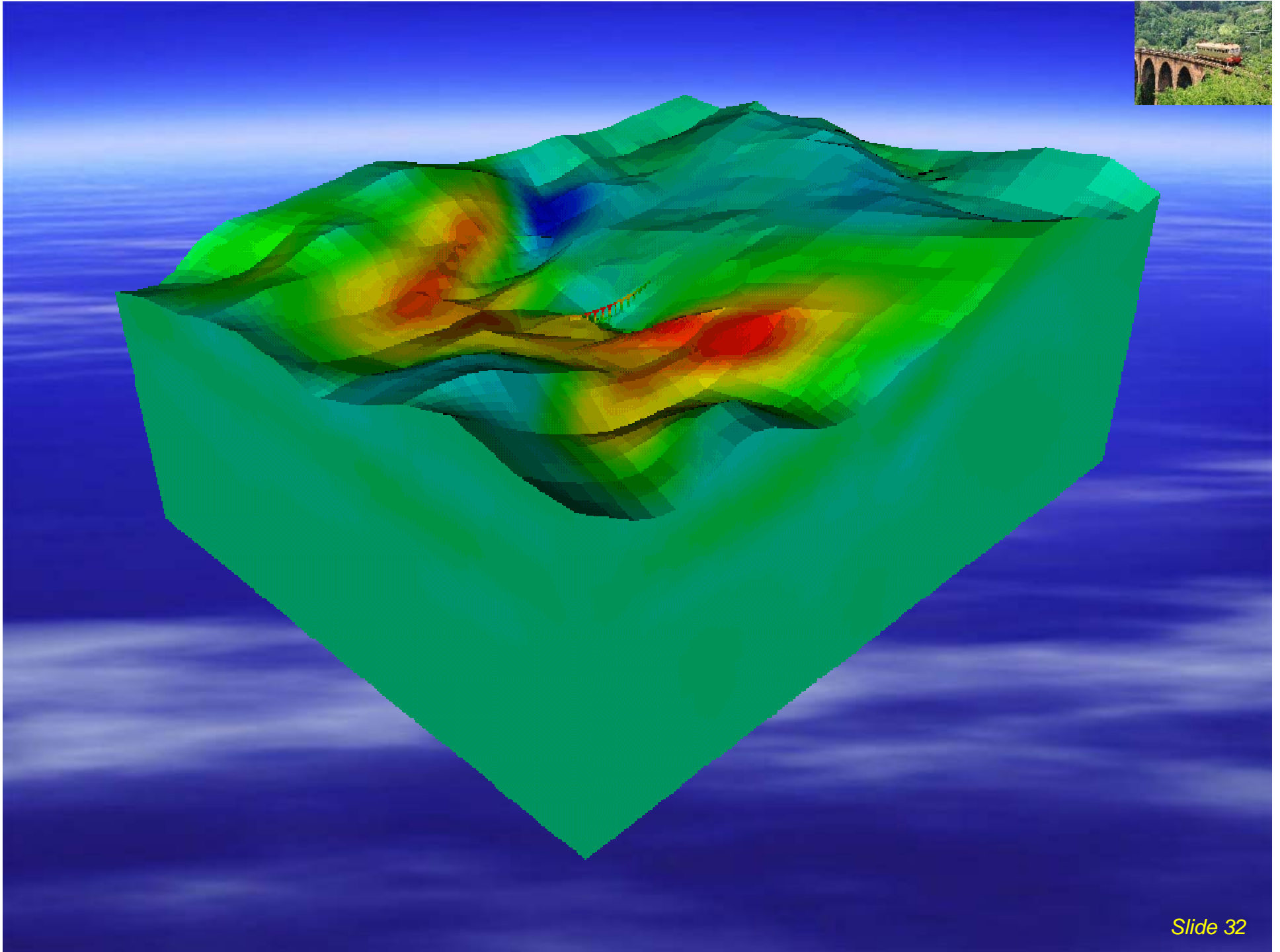
Alluvial Deposits  
(max depth 30 m) with a  
masonry railway bridge  
crossing the valley













# *Turkey earthquake 1999*



## ***GEO-ELSE Viscoplastic***

⇒ Elastoviscoplastic constitutive model

⇒ Total strain tensor can be written as:

$$\dot{\varepsilon}_{ij} = \dot{\varepsilon}_{ij}^{el} + \dot{\varepsilon}_{ij}^{vp}$$

and constitutive behaviour is given by:  $\frac{\partial \sigma}{\partial t} = D^e : \left( \frac{\partial \varepsilon}{\partial t} - \frac{\partial \varepsilon^{vp}}{\partial t} \right)$

⇒ The flow rule can be written as below

$$\frac{\partial \varepsilon^{vp}}{\partial t} = \gamma m \langle \phi(f) \rangle \quad (\text{Perzyna, 1963})$$

where  $f$  is the yield function,  $\phi(f)$  is the viscous nucleus,  $\gamma$  is a parameter which describes the system evolution rate,  $m$ , is the gradient to the plastic potential  $m = \partial g / \partial \sigma_{ij}$ , and characterizes the direction of viscoplastic strain.

## *GEO-ELSE Viscoplastic*

⇒ Viscous nucleus definition:

$$\phi(f) = \left( \frac{f - f_0}{f_0} \right)^N$$

where  $f_0$  is the value characterizing the stress level below which no flow occurs, N is constitutive parameter

⇒ Von Mises o Drucker-Prager yield surface

⇒ The size of the yield surface varies according to a suitable softening law (linear dependence on the equivalent deviatoric viscoplastic strain  $\bar{\varepsilon}^{vp}$  )



# *Implementation of the viscoplastic model in GEO-ELSE*

For simplicity absence of absorbent conditions and not viscous material

$$[M] \ddot{u}(t) + [K] u(t) = F_{ext}(t)$$

with  $[K] u(t) = F_{int} = \int_{\Omega} \sigma d\Omega$

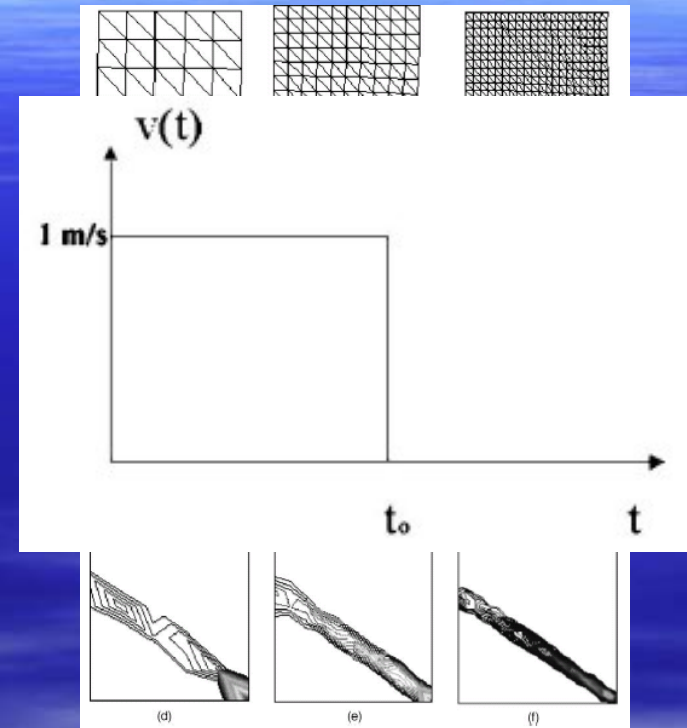
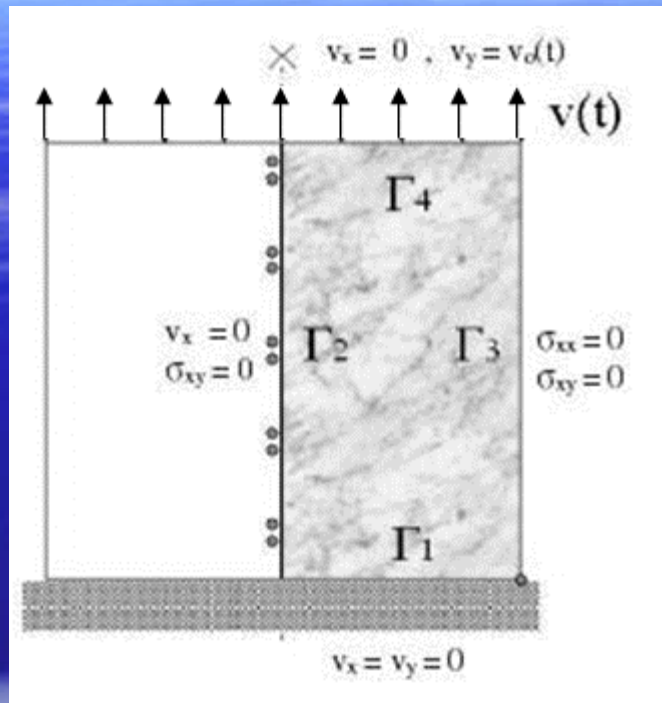
$$\sigma_n = D^e : \left( \varepsilon_n - \varepsilon_n^{vp} \right)$$

The viscoplastic model introduces an additional right hand side term

$$u_{n+1} = (F_{ext} - F_{int}) [M]^{-1} \Delta t^2 + 2u_n - u_{n-1}$$



# 1-Simulation of the shear band creation



$$\bar{\varepsilon}^{vp} = \sqrt{\frac{2}{3}} \left\{ \left( \varepsilon_{ij}^{vp} \right) \left( \varepsilon_{ij}^{vp} \right) \right\}^{1/2}$$

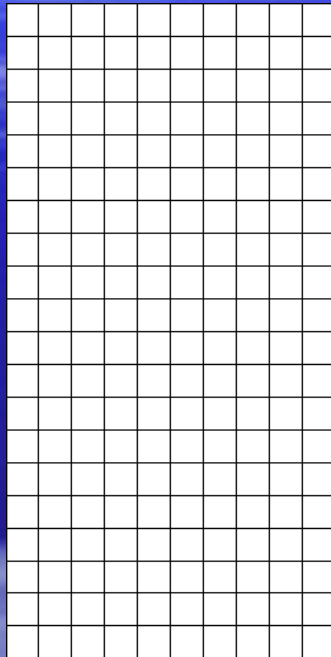
M. Mabssout, M. Pastor

“A Taylor-Galerkin algorithm for shock wave propagation and strain localization failure of viscoplastic continua”

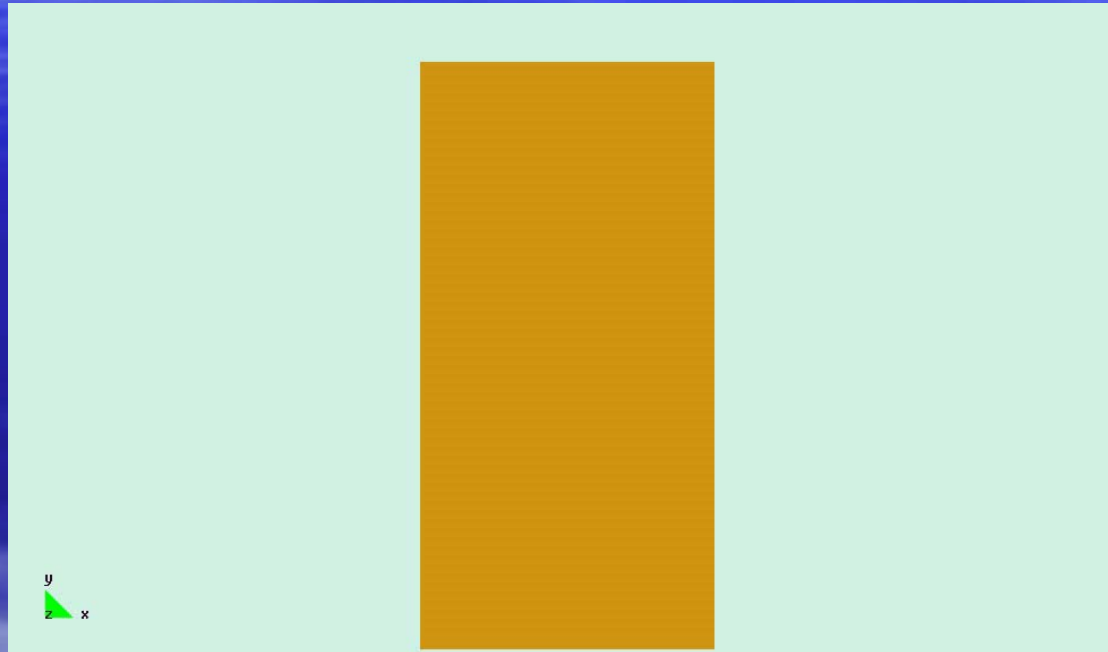
Comput. Methods Appl. Mech. Engrg., 2003

# *1-Simulation of the shear band creation*

Mesh



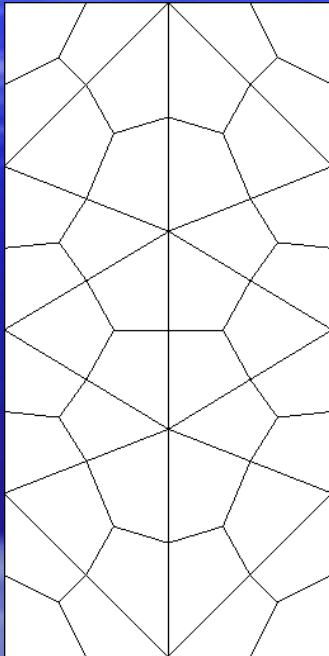
Spectral degree = 4, Dt = 10% Dt<sub>CFL</sub>



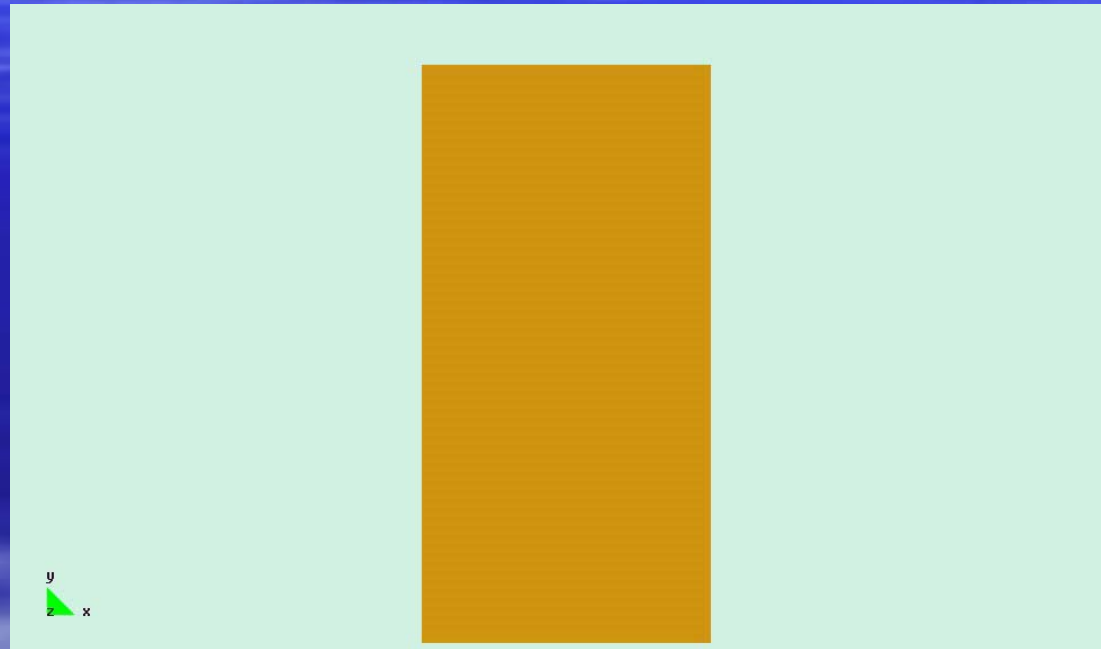
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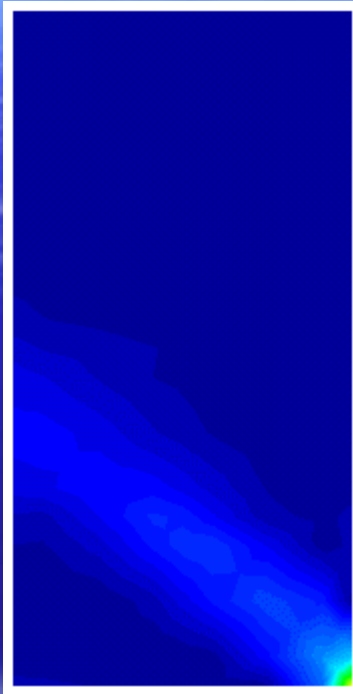
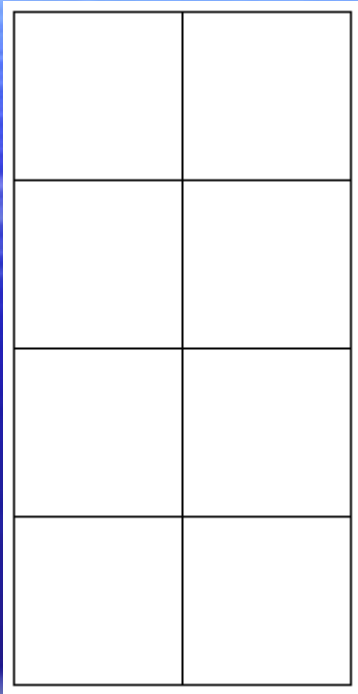


Spectral degree = 10, Dt = 10% Dt<sub>CFL</sub>

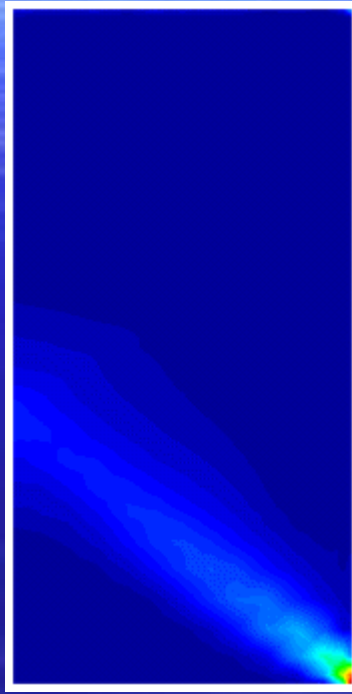


$$\bar{\varepsilon}^{vp} = \sqrt{\frac{2}{3}} \left\{ \left( \varepsilon_{ij}^{vp} \right) \left( \varepsilon_{ij}^{vp} \right) \right\}^{1/2}$$

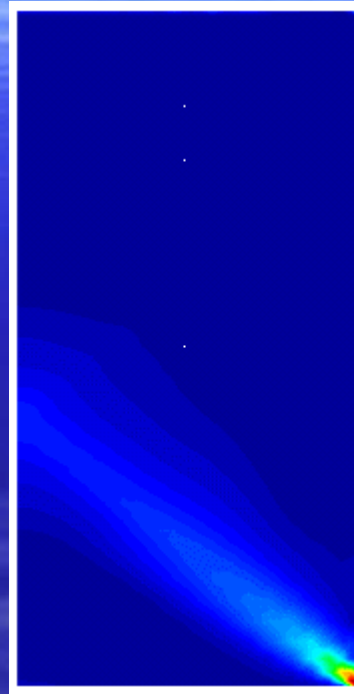
# *1-Simulation of the shear band creation*



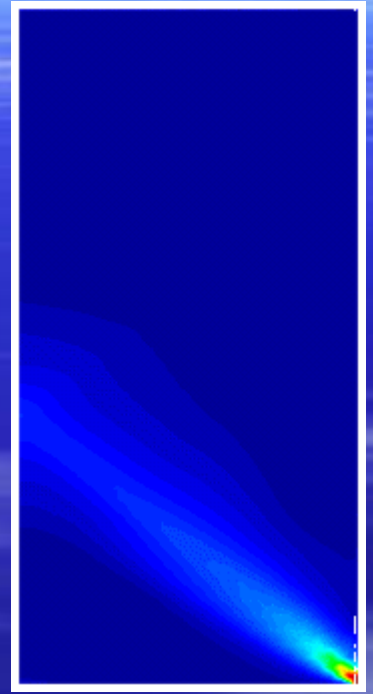
SD = 4,  
Nodes = 153



SD = 8,  
Nodes = 561



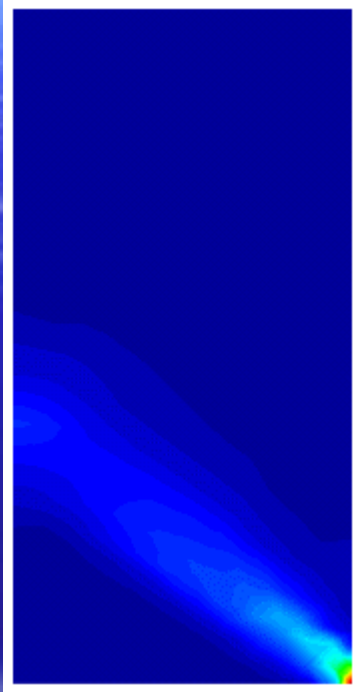
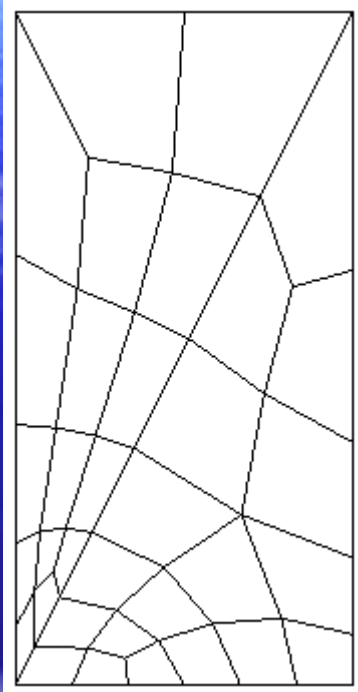
SD = 12,  
Nodes = 1225



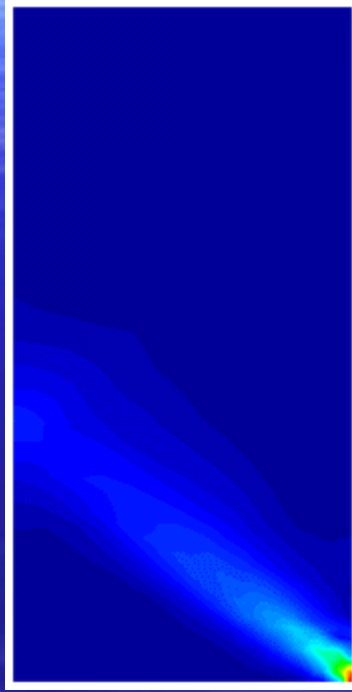
SD = 16,  
Nodes = 2145



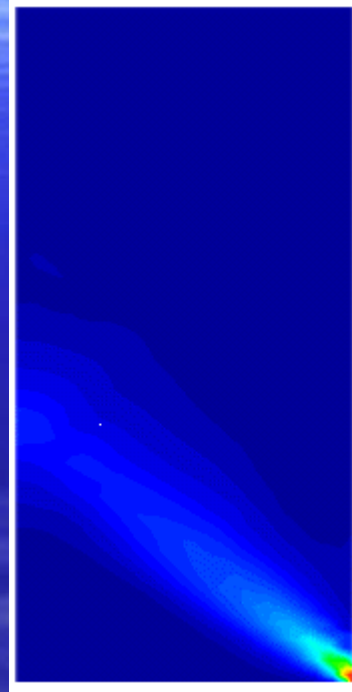
# *1-Simulation of the shear band creation*



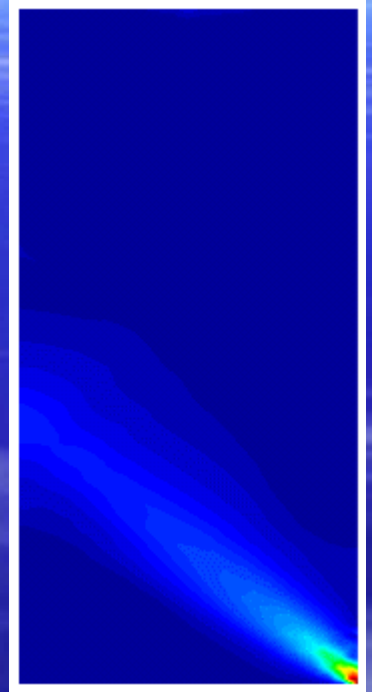
SD = 3,  
Nodes = 334



SD = 4,  
Nodes = 581



SD = 5,  
Nodes = 896



SD = 6,  
Nodes = 1279

## 2-Soil structure - hazard analysis

Alluvial valley:

$$\begin{aligned}V_p &= 300 \text{ m/s} \\V_s &= 150 \text{ m/s} \\ \rho &= 1800 \text{ kg/m}^3\end{aligned}$$

Bedrock:

$$\begin{aligned}V_p &= 1000 \text{ m/s} \\V_s &= 600 \text{ m/s} \\ \rho &= 2500 \text{ kg/m}^3\end{aligned}$$

90 m

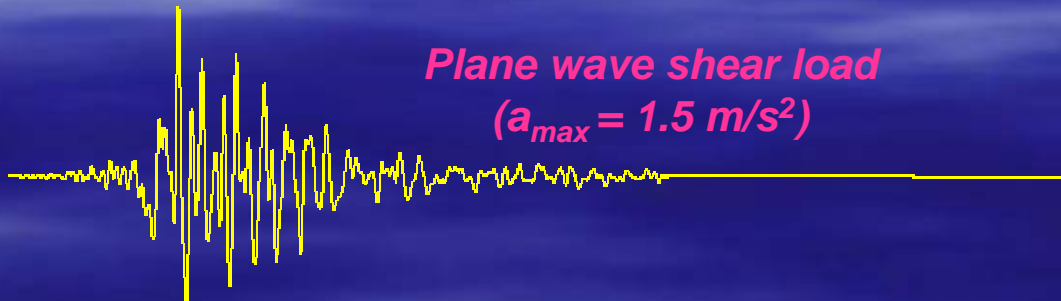


15 m

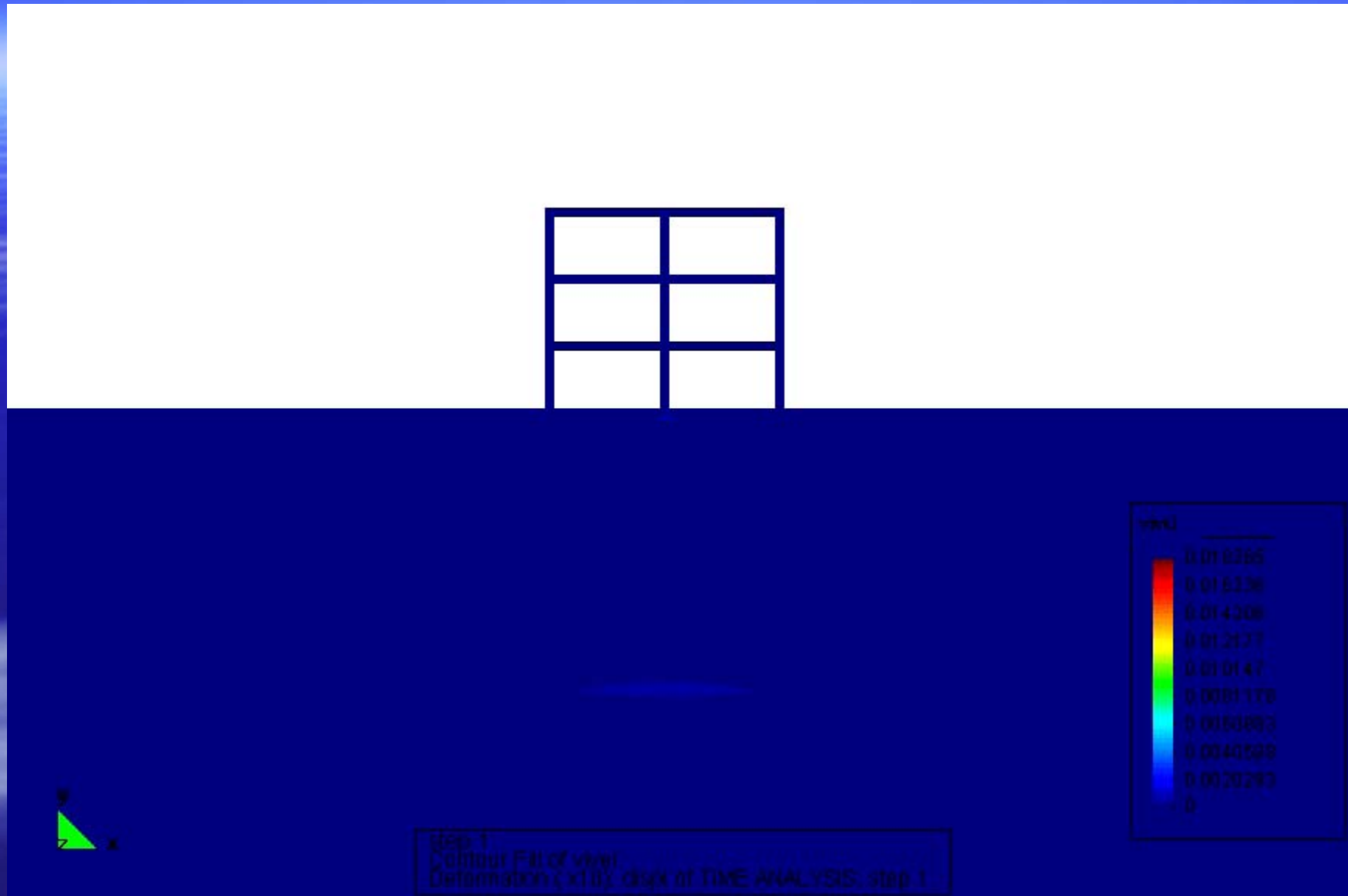
ABC

500 m

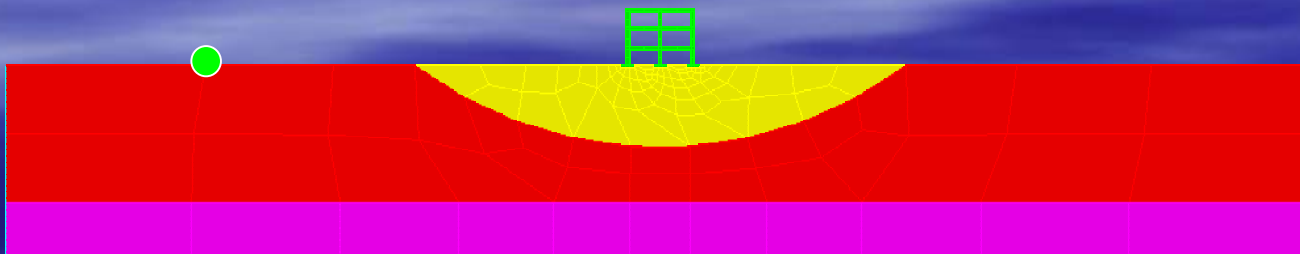
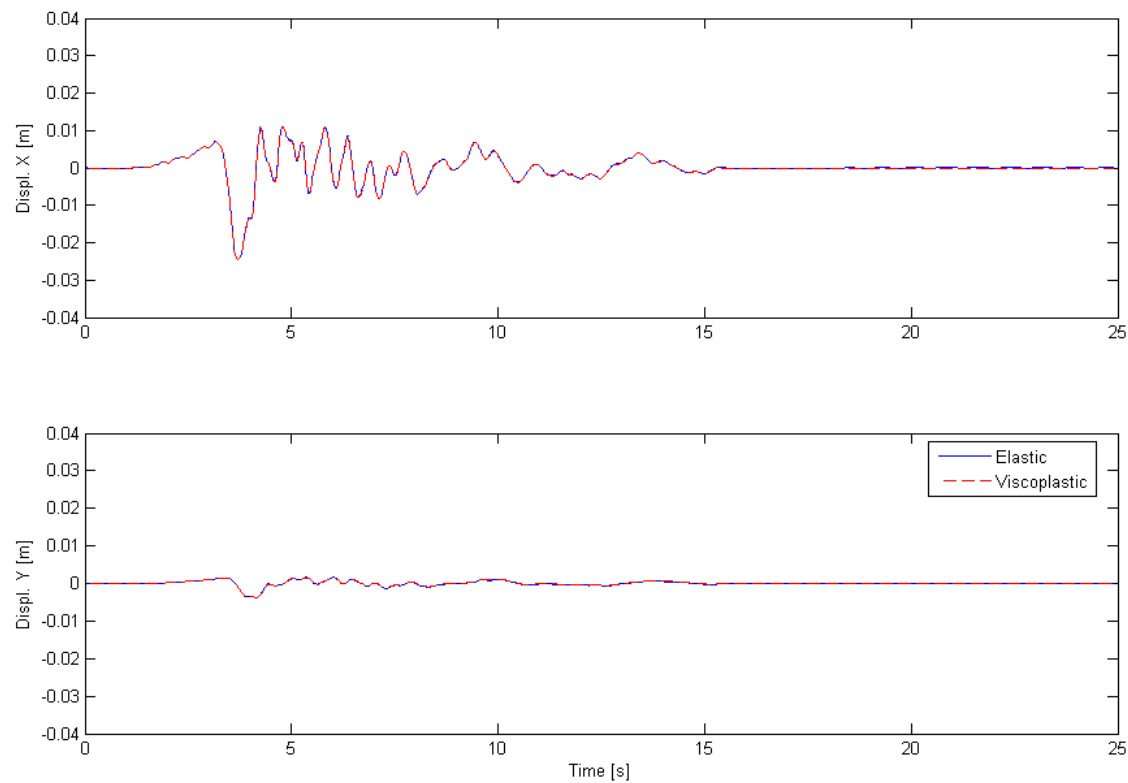
Plane wave shear load  
( $a_{\max} = 1.5 \text{ m/s}^2$ )



## *2-Soil structure - hazard analysys*

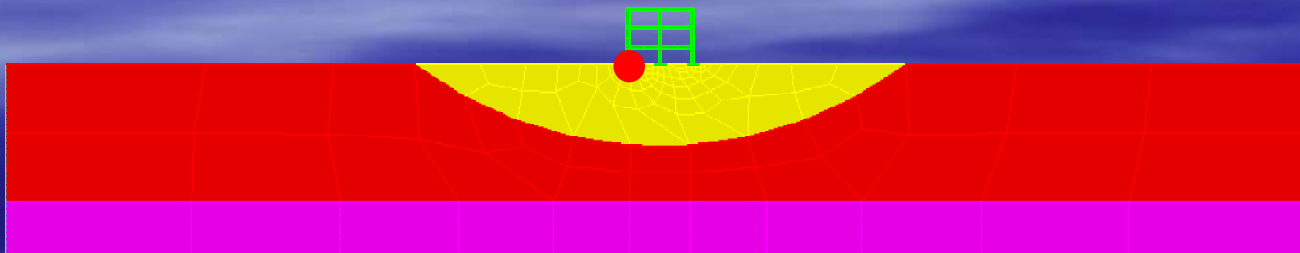
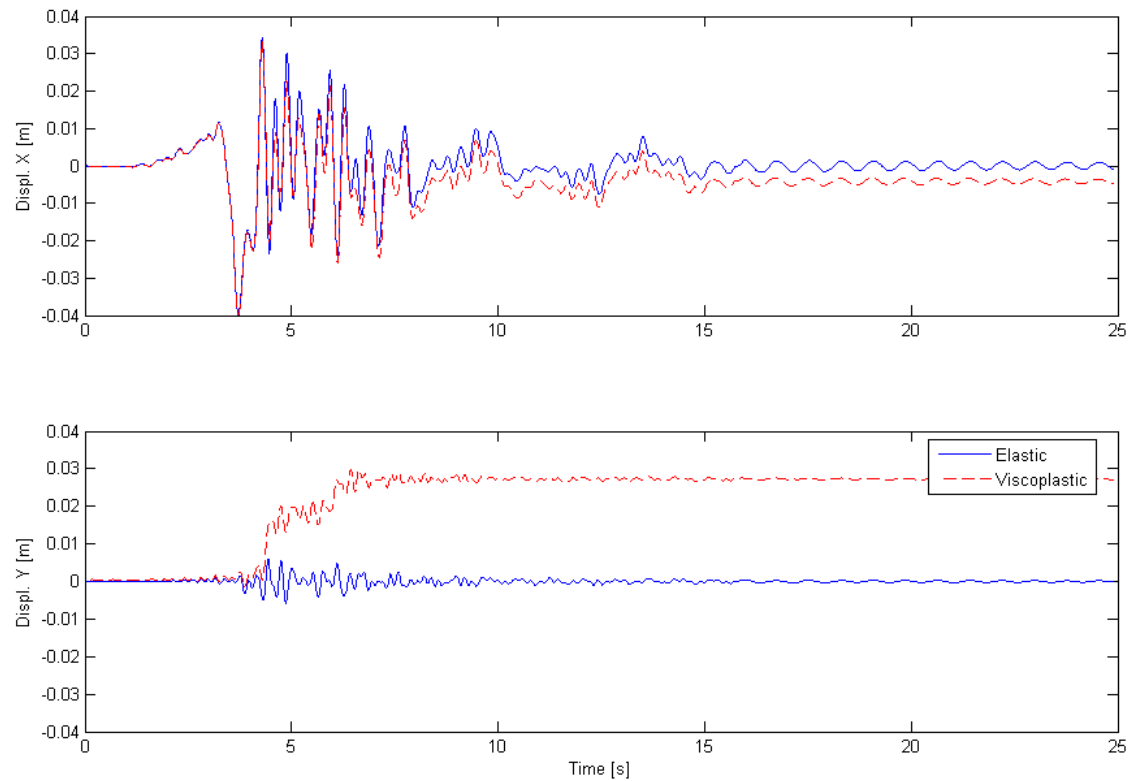


## 2-Soil structure - time histories





## *2-Soil structure - time histories*



***THE END***